

5.4-15. Suggestion: Use the 'Beam Deformation' option of the "General Analysis Module" of MDSolids to check your solution.

5.4-17. Suggestion: Use the 'Beam Deformation' option of the "General Analysis Module" of MDSolids to check your solution.

$$\mathbf{5.4-19.} \quad V(x) = -p_0 L \left[\frac{2}{3} - \left(\frac{x}{L} \right) + \frac{1}{3} \left(\frac{x}{L} \right)^3 \right],$$

$$M(x) = p_0 L^2 \left[\frac{1}{4} - \frac{2}{3} \left(\frac{x}{L} \right) + \frac{1}{2} \left(\frac{x}{L} \right)^2 - \frac{1}{12} \left(\frac{x}{L} \right)^4 \right]$$

5.5-1. Use the "Determinate Beams Module" of MDSolids to check your solution.

5.5-3. Use the "Determinate Beams Module" of MDSolids to check your solution.

5.5-5. Use the "Determinate Beams Module" of MDSolids to check your solution.

5.5-7. Use the "Determinate Beams Module" of MDSolids to check your solution. (Hint: Let $P = 1$ kip, and let $a = 1$ ft.)

5.5-9. Use the "Determinate Beams Module" of MDSolids to check your solution.

5.5-11. Use the "Determinate Beams Module" of MDSolids to check your solution.

5.5-13. Use the "Determinate Beams Module" of MDSolids to check your solution.

5.5-15. Use the 'Beam Deformation' option of the "General Analysis Module" of MDSolids to check your solution.

5.5-17. Suggestion: Use the "Determinate Beams Module" of MDSolids to check your solution. Enforce symmetry by using only the left (or right) half of the beam.

5.5-19. $M_1 = (3x_1)$ kip·ft, $M_2 = 3(5 - x_2)$ kip·ft

5.6-1 – 5.6-21. Section 5.6 is an optional section.

CHAPTER 6

6.2-1. (a) $\epsilon_{\max} = 0.001041 \frac{\text{mm}}{\text{mm}}$,

(b) $\epsilon_{\max} = 0.001664 \frac{\text{mm}}{\text{mm}}$

6.2-3. (a) $L_{\min} = 1.571$ m, (b) $\epsilon_{\max} = 1.571(10^{-3}) \frac{\text{mm}}{\text{mm}}$

6.2-5. $\kappa' = \nu \kappa$

6.3-1. $\sigma_{\max} = \frac{Eh}{(2r + h)}$

6.3-3. $h_{\max} = 0.0497$ in.

6.3-5. (a) $\sigma_{\max T} = 69.4$ MPa, (b) $\rho = 181.4$ m,

(c) $\delta_{\max} = 44.1$ mm

6.3-7. (a) $\sigma_A = -1700$ psi, (b) $\sigma_C = 1400$ psi, $F_{\text{top}} = 11.55$ kips (C)

6.3-9. $(M_z)_{\max} = 220$ kN·m, (b) $(M_y)_{\max} = 75.6$ kN·m

6.3-11. $\sigma_A = -2.16$ ksi, $\sigma_B = 4.20$ ksi

6.3-13. (a) $\bar{\eta} = \frac{23a}{7}$, (b) $\sigma_{\max T} = \frac{483}{12,796} \frac{M}{a^3}$

(c) $F_{\text{top}} = \frac{108}{457} \left(\frac{M}{a} \right)$ (C)

6.3-15. $(\sigma_B)_{\max} = 16.23$ ksi

6.3-17. $(\sigma_C)_{\max T} = 37.3$ MPa

6.3-19. $(\sigma_B)_{\max T} = 9.28$ ksi, (b) $(\sigma_C)_{\max C} = -10.64$ ksi

6.3-21. (a) Use the "Determinate Beams Module" of MDSolids to check your solution for Part (a).

(b) $\sigma_{\max} = 12.52$ ksi

6.3-23. $(\sigma_A)_{\max T} = 787$ psi, $(\sigma_A)_{\max C} = -2910$ psi

6.3-25. (a) Use the "Determinate Beams Module" of MDSolids to check your solution for Part (a).

(b) $\sigma_{\max} = 3.14$ ksi

6.3-27. (a) Use the "Determinate Beams Module" of MDSolids to check your solution for Part (a).

(b) $\sigma_{\max} = 18.00$ MPa

6.3-29. (a) Use the "Determinate Beams Module" of MDSolids to check your solution for Part (a).

(b) $\sigma_{\max} = 11.18$ ksi

6.3-31. (a) Use the "Determinate Beams Module" of MDSolids to check your solution for Part (a).

(b) $\sigma_{\max} = 35.8$ MPa

6.3-33. (a) Use the "Determinate Beams Module" of MDSolids to check your solution for Part (a).

(b) $\sigma_{\max T} = 1.457$ ksi $\sigma_{\max C} = -1.165$ ksi

$\sigma_{\max T}$ occurs at $x = 3$ ft; $\sigma_{\max C}$ occurs at $x = 6$ ft.

6.3-35. Letting $w = 1$ kN/m, use the "Determinate Beams Module" of MDSolids to check your solution for $M(x)$. $w_{\max} = 108.1$ kN/m

6.3-37. (a) $\frac{\sigma_{\max}}{\sigma_{y\max}} = 3 \left(\frac{x}{h} \right)^2$, (b) —

6.3-39. Use MDSolids.

6.3-41. Use MDSolids.

6.4-1. $W16 \times 40$, ($S = 64.7$ in³)

6.4-3. $S_{\text{design}} = 1667(10^3)$ mm³, $W 610 \times 92$

6.4-5. (a) $S = \frac{b(nt)^2}{6}$, (b) $n = 5$ planks

6.4-7. $b_{\min} = 230$ mm

6.4-9. $S_{\text{design}} = 36.0$ in³, $W 12 \times 35$

6.4-11. $b = \frac{\sqrt{3}}{6} d$, $h = \frac{\sqrt{6}}{3} d$

6.4-13. (a) $b = 0.838$ m,

(b) $S_{\text{design}} = 314(10^3)$ mm³, $W 310 \times 33$

6.4-15. $P_{\text{allow}} = 1.692$ kips

6.5-1. $(\sigma_{\max})_w = 509$ psi, $(\sigma_{\max})_s = 11.20$ ksi

6.5-3. $P_{\text{allow}} = (P_{\text{allow}})_s = 57.7$ kN

6.5-5. $(\sigma_{\max})_a = \frac{24M}{bh^2(1+7n)}$, $(\sigma_{\max})_b = \frac{48Mn}{bh^2(1+7n)}$